

	Project The House	Project ref 777
	Calcs for Steel Beam	Date

## Steel Beam Design

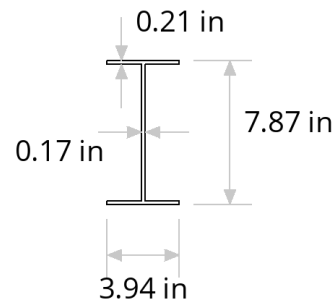
\* You can add your own text, diagrams or photos here \*

### Design summary

	Resistance / Limit	Applied / Actual	Utilization	
Shear resistance (kip)	19.3	4.35	<b>23 %</b>	<b>OK</b>
Bending resistance (kip-ft)	15.5	13.1	<b>84 %</b>	<b>OK</b>
Total deflection (in)	0.6	0.38	<b>63 %</b>	<b>OK</b>
Live deflection (in)	0.4	0.27	<b>68 %</b>	<b>OK</b>

### Beam details

Beam shape	<b>W 8 x 10</b>
Effective span	<b>12 ft</b>
Minimum yield stress $F_y$	<b>36,000 psi</b>
Width	<b>3.94 in</b>
Depth	<b>7.87 in</b>
Web	<b>0.17 in</b>
Flange	<b>0.21 in</b>
Weight per foot	<b>10.08 lb/ft</b>
Modulus of elasticity	<b>29,000 ksi</b>
Second moment of area	<b>30.75 in<sup>4</sup></b>



### Lateral bracing & deflection limits

Beam is laterally braced along its length  
Length between lateral bracing at least every **2 ft**

Live load deflection limit: **span / 360.00 = 0.40 in**  
Total load deflection limit: **span / 240.00 = 0.60 in**

### Loading details



#### Self weight

Dead load

**10.08 lb/ft**



#### Load 1: UDL - Residential Floor

Dead load

**15 psf × 13 ft = 195 lb/ft**

Live load

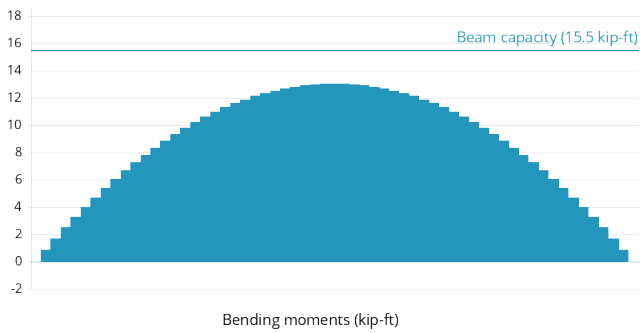
**40 psf × 13 ft = 520 lb/ft**

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### Reactions (unfactored)

	Dead	Live	Total
Left reaction	<b>1.23 kip</b>	<b>3.12 kip</b>	<b>4.35 kip</b>
Right reaction	<b>1.23 kip</b>	<b>3.12 kip</b>	<b>4.35 kip</b>

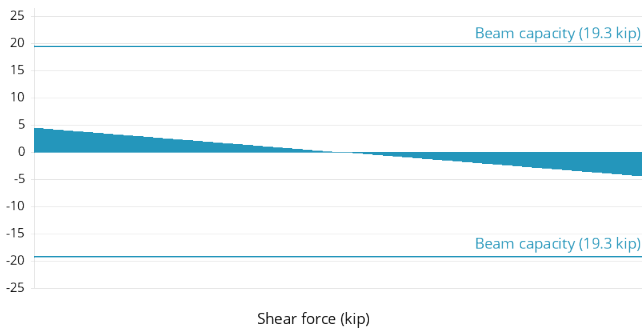
### Check bending moments



**Beam moment capacity  $M_r = 15.5 \text{ kip-ft} \geq 13.1 \text{ kip-ft}$ , therefore OK**

The top flange of the beam is to be laterally braced along its full length. To ensure adequate lateral bracing, bracing members should be attached with fasteners that provide a positive connection. Lateral bracing members should generally be regularly spaced at least every 2 feet.

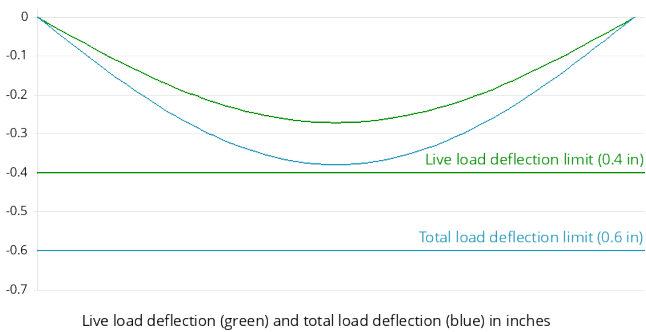
### Check shear force



**Shear capacity  $V_c = 19.3 \text{ kip} \geq 4.35 \text{ kip}$ , therefore OK**

Allowable shear =  $0.4 \times \text{minimum yield stress} \times d \times t_w$

### Check deflection



**Live load deflection = 0.27 in  $\leq 0.4$  in, therefore OK**

**Total load deflection = 0.38 in  $\leq 0.6$  in, therefore OK**

### Notes

These calculations are based on the Manual of Steel Construction, Allowable Stress Design, Ninth Edition by the American Institute of

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